

FOR

TAMUNING - DEDEDO
AGANA
ASAN
PITI
MERIZO
AGAT-SANTA RITA



SCO INTERNATIONAL ARCHITECTS / ENGINEERS

LOS ANGELES

GUAM

SAIPAN

HONOLULU

VIRGINIA BEACH

August 1970

Juan C. Tenorio
Director of Public Works
Government of Guam
Tumon, Guam

Dear Mr. Tenorio,

In accordance with an agreement made on October 16, 1968 between the Government of Guam and ESCO International, we hereby submit a report entitled, "Storm Drainage Master Plan."

This study is the result of hydrologic and hydraulic analysis performed while giving consideration to probable future land development. The proposed drainage system will provide comprehensive storm drain control for:

- 1. Tamuning Dededo
- 2. Agana
- 3. Asan
- 4. Piti
- 5. Agat Santa Rita
- 6. Merizo

Functional plans and estimate presented in this report for storm drainage improvements are based upon the best data now available; however, changes in land use, street improvements, utility locations, or information developed by more detailed studies require adjustments when final designs and plans are prepared.

We appreciate the opportunity to participate in the development of the territory of Guam. The cooperation and assistance of your office has been highly beneficial and gratifying.

Respectfully submitted,

Michael D. Flynn, P.E.

Manager, Guam Office

SUMMARY

The rapid growth of the municipal areas of the territory of Guam coupled with its pluvial climate necessitates the development of an adequate drainage system for the preservation of municipal and private property. It is essential that a comprehensive and orderly program of storm drainage facilities be provided to insure orderly future development within these areas.

The following objectives are set forth in order that this orderly development may be effected:

- 1. Provide a system of storm drain facilities based on rainfall resulting from the selected design storm that will:
 - A. Dispose of storm waters without damage to municipal or private property.
 - B. Permit normal business and traffic to continue.
 - C. Alleviate surface water flow as a nuisance to the public.
- 2. Provide a comprehensive plan which will insure maximum use of existing facilities, whenever available and adequate, and which will adapt to economical future development.

The plan establishes drainage systems for each of the six subject areas which will permit adequate disposal of storm water. Whenever possible, the natural course of drainage flow was adhered to. The proposed plans provide a general routing of all channels and covered drains, and provide for the accomplishment of the construction work in such order or procedure as may be required to afford the greatest relief to the greatest number of people consistent with good engineering practice.

The improvements are based upon ultimate requirements for capacity and a permanent type of construction. However, construction of all the units on this basis at the present time is not economically justified. Three proposals are presented for the accomplishments of the proposed constructions.

- 1. Underground conduits, where required, be constructed to ultimate capacity possibly under highway funds.
- Open channels in unimproved areas be constructed permanently, possible with funds contributed by owners or developers of adjacent areas being served by the respective facility.
- 3. Open channels in areas, which have already reached their full design development, be constructed permanently, with funds from special assessment.

It is not contemplated that all storm drains in the report be installed immediately or concurrently. The drainage works proposed are considered to provide a pattern of flood protection which will facilitate orderly development of unimproved areas and sound redevelopment of renewal of improved areas.

"Sinks" of various sizes have been taken into consideration in this report. However it should be noted that these local depressions alone cannot, in most instances, contain the volume of storm water generated from large drainage areas.

The percolation rate in the Harmon-Dededo area is estimated at 1" in 5 minutes. Under this condition the "Sinks" are adequate for low rainfall intensity only. Consequently only major "Sinks" which cannot be economically developed should be retained.

HYDROLOGY AND HYDRAULICS

The method of hydrologic computation were based on the "Storm Drainage Standards and Design Criteria," a study prepared by Austin, Smith and Associates, Inc., Agana Guam, for the Government of Guam. The design storm or recurrence interval, run-off coefficient, time of concentration and rainfall intensity were computed as outlined in that report. The method of tabulation was devised by ESCO INTERNATIONAL for this report. The rational method of hydrologic calculation, as modified by Austin, Smith, was used in all the areas and lines studied, except in the case of Line F. Merizo, was used. All areas within the project boundaries were considered to be fully developed.

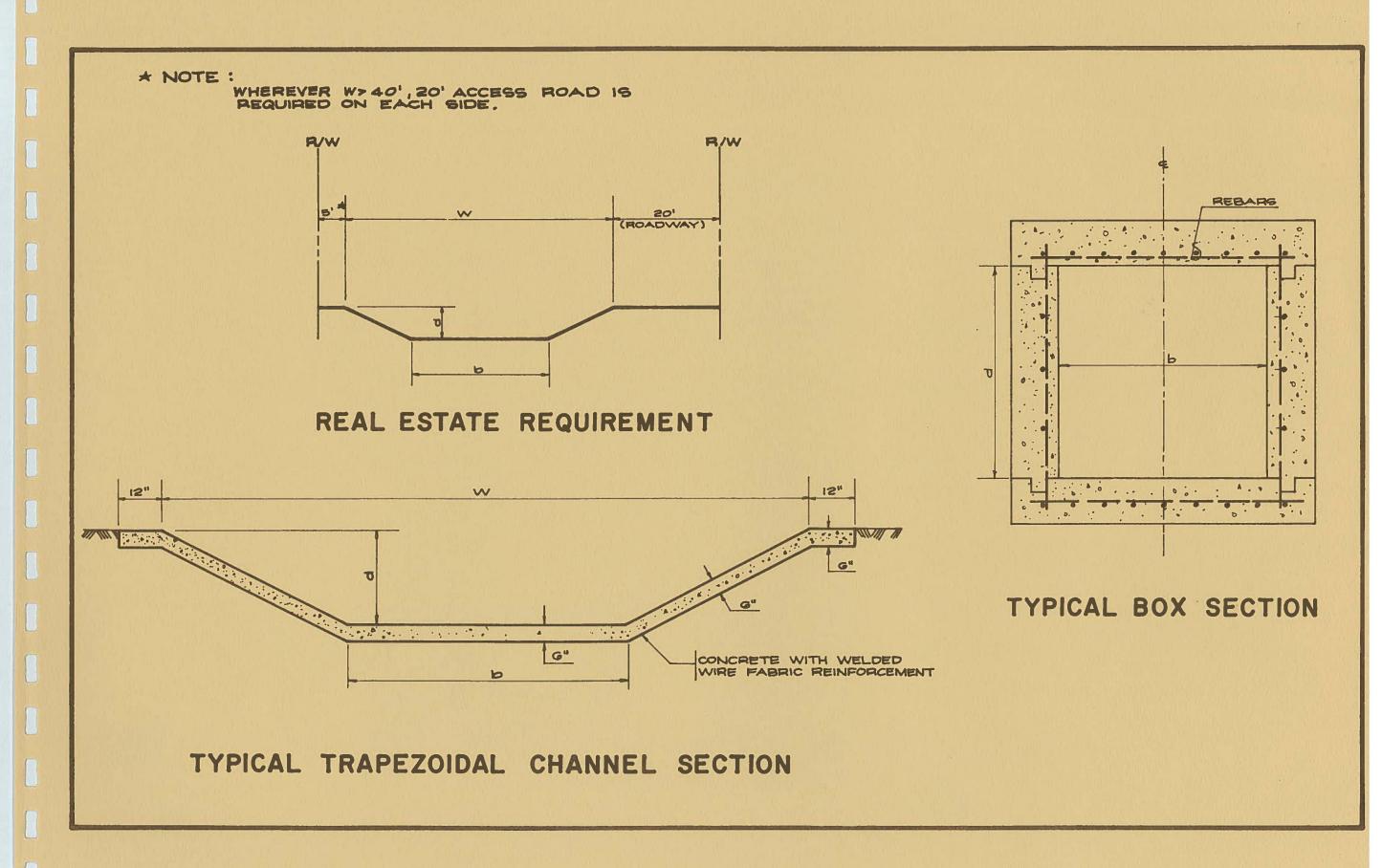
Nomographs, developed by the U.S. Public Road Administration, Washington, D. C., were used in sizing channels from bottom widths (b) of 2-20 feet. Channels, with (b) greater than 20 feet, were sized using HANDBOOK OF HYDRAULICS by King and Brater.

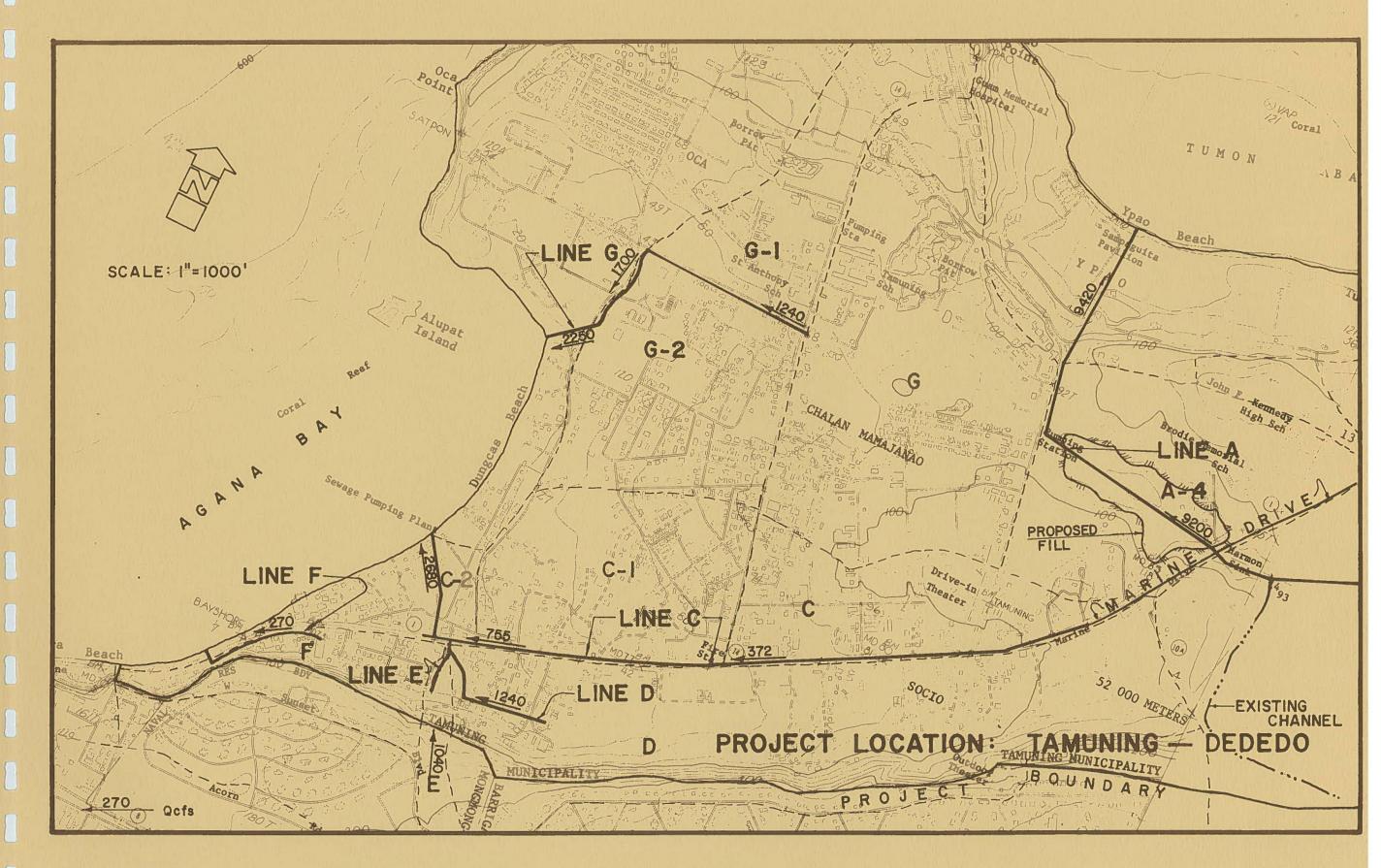
Rectangular channels (box) and trapezoidal channels were sized, allowing for a freeboard as outlined in Austin, Smith's Report. Pipes

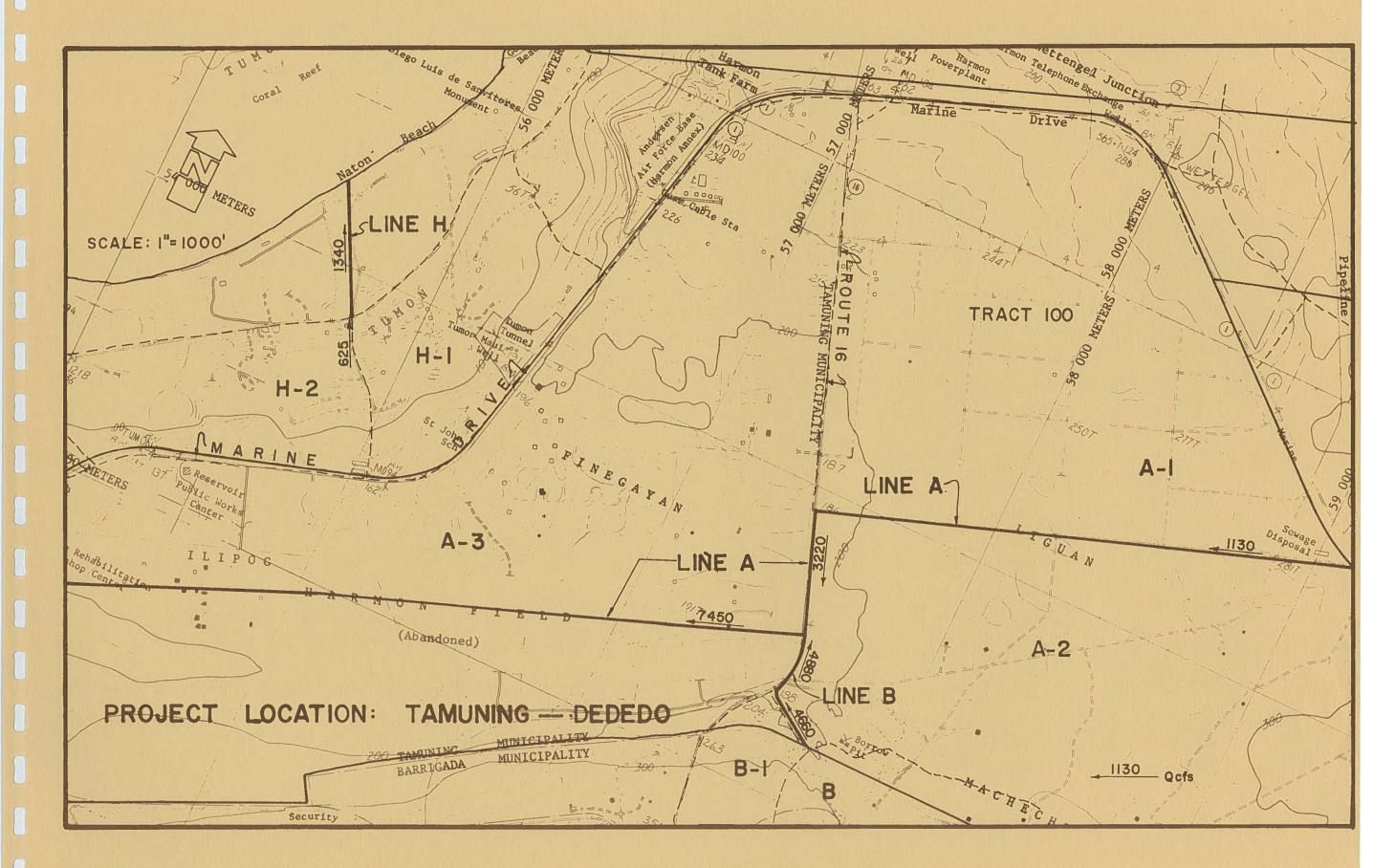
were sized such that the friction slope is equal to the slope of the pipe, or stated differently, the pipe is just flowing full or nearly full. Pipes are used only on small areas, hence the reason for their flowing full.

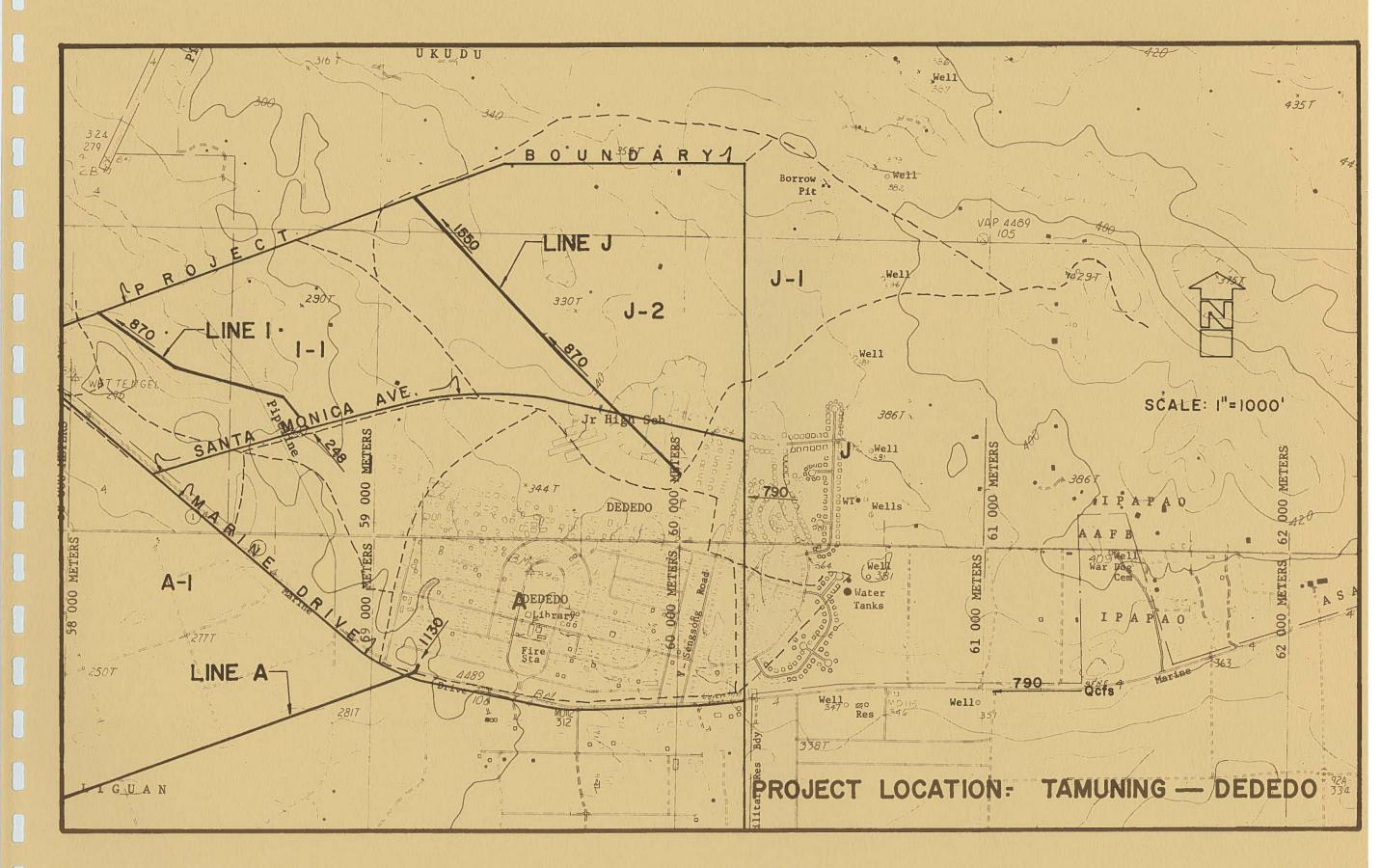
For the purposes of this report a trapezoidal channel section was used, however in the detail planning and preparation of final drawings other sections may be used as dictated by factors beyond our immediate control. Erosion control, velocity dissipators, debris guards, etc. are not included in this report.

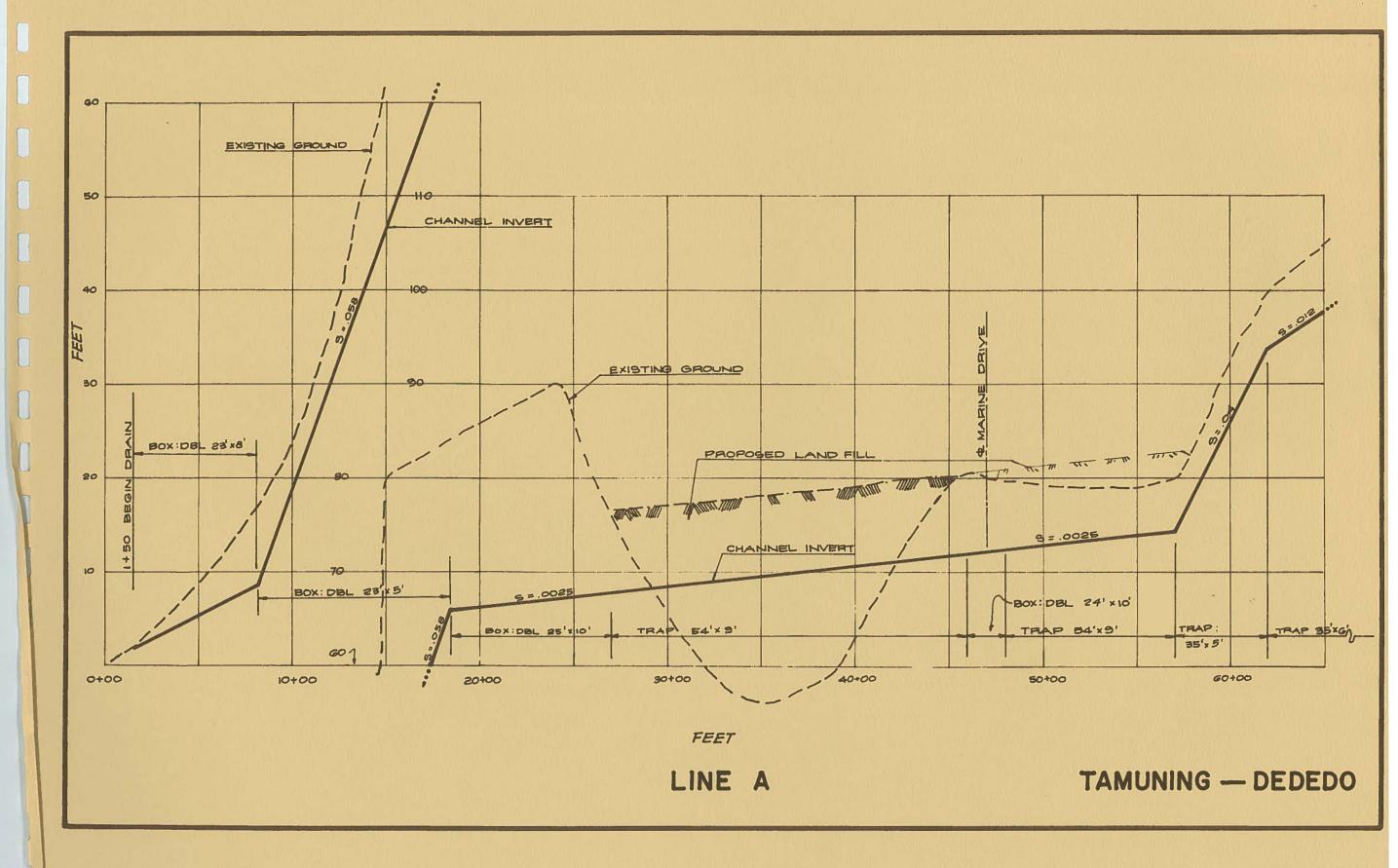
The following explanation is in order. The designation "Box: 3x2" means the section is of the rectangular variety with a concrete cover, having inside dimension of 3 feet wide and 2 feet high. The designation "Trap: 3x2" indicates a trapezoidal variety, having a bottom width (b) of 3 feet and a depth (d) of 2 feet. In all cases, trapezoidal channels have side slopes of 2 horizontal to 1 vertical. "RCP: 36" diameter" indicates a reinforced concrete pipe, having inside diameter of 36".

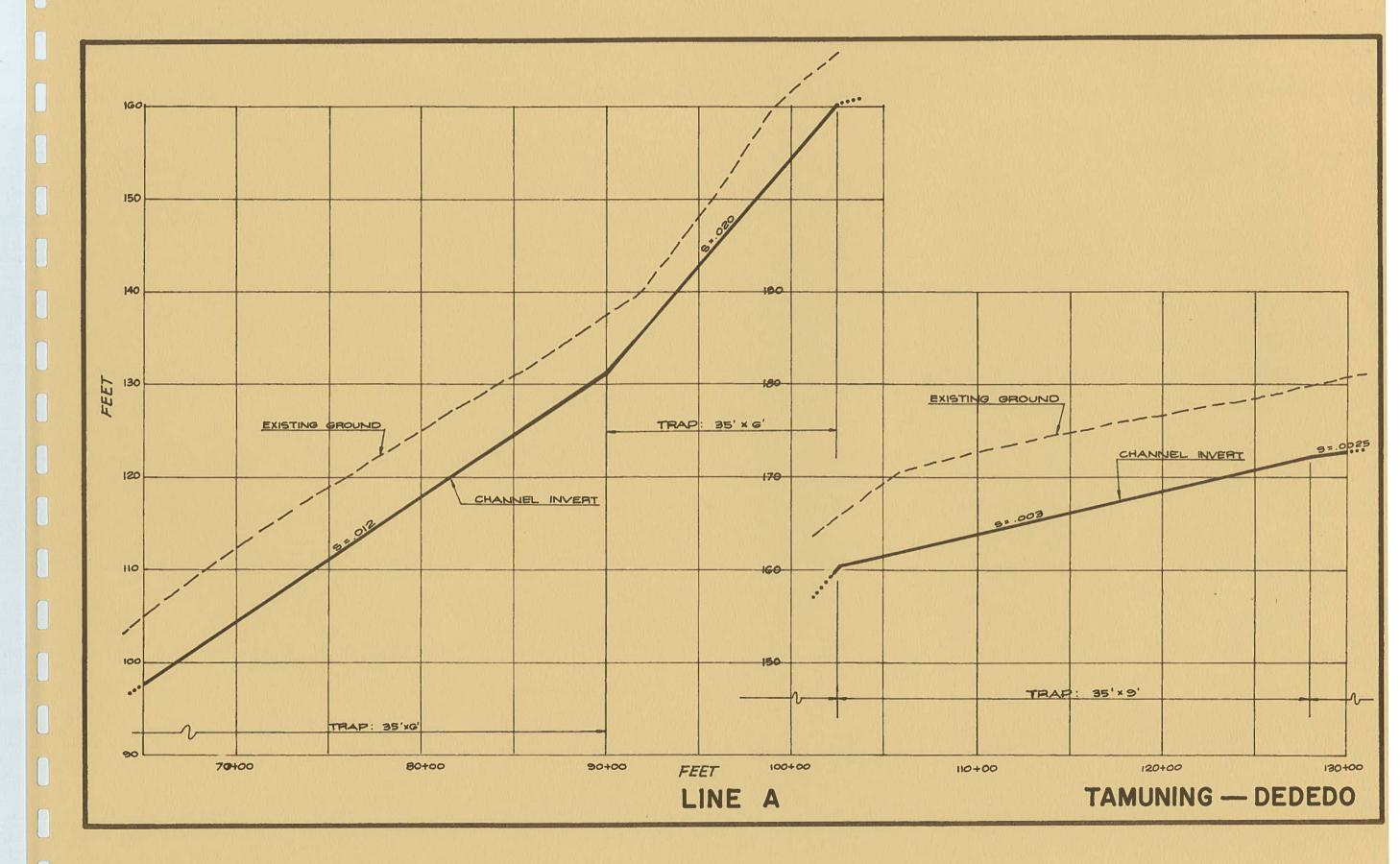


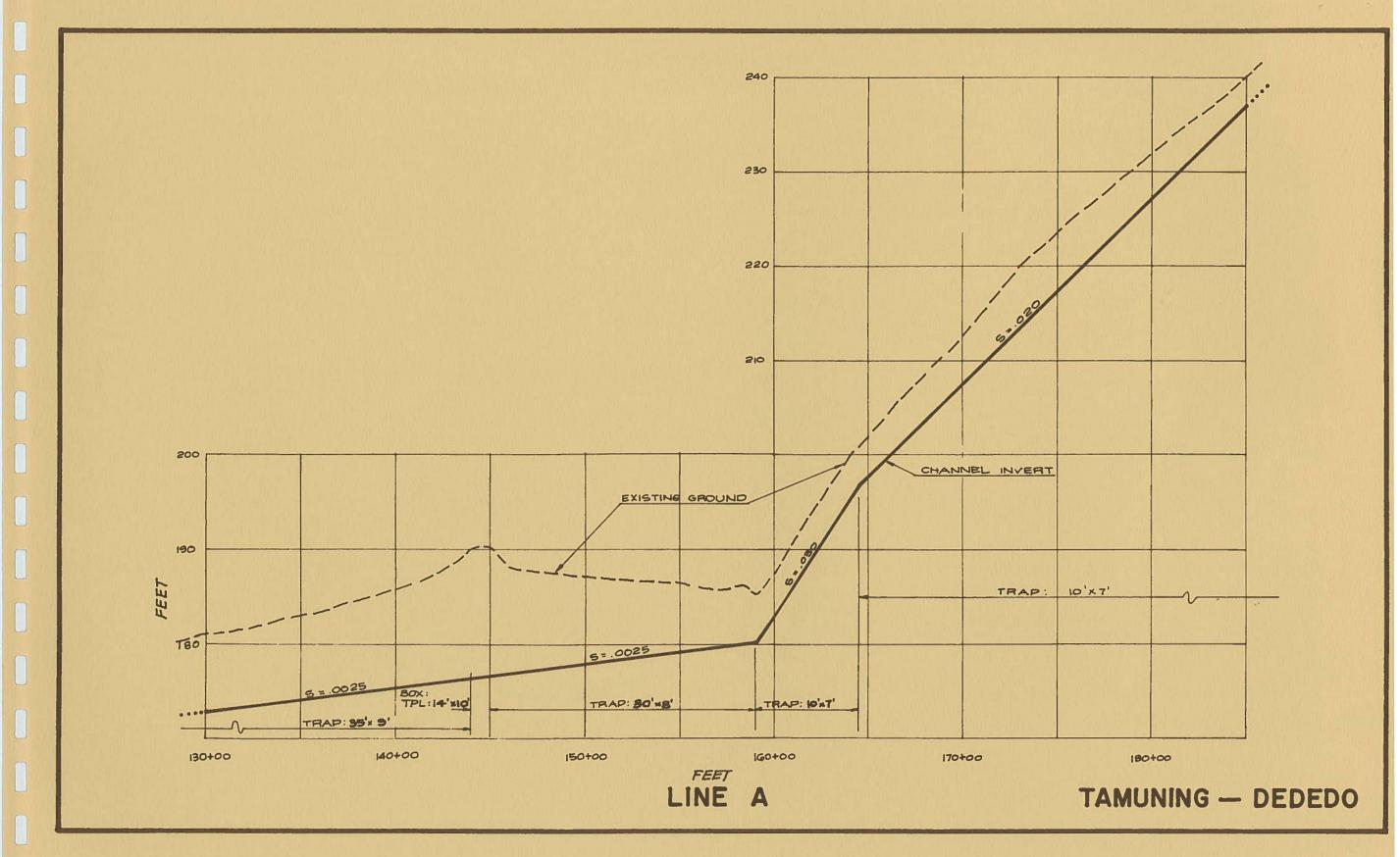


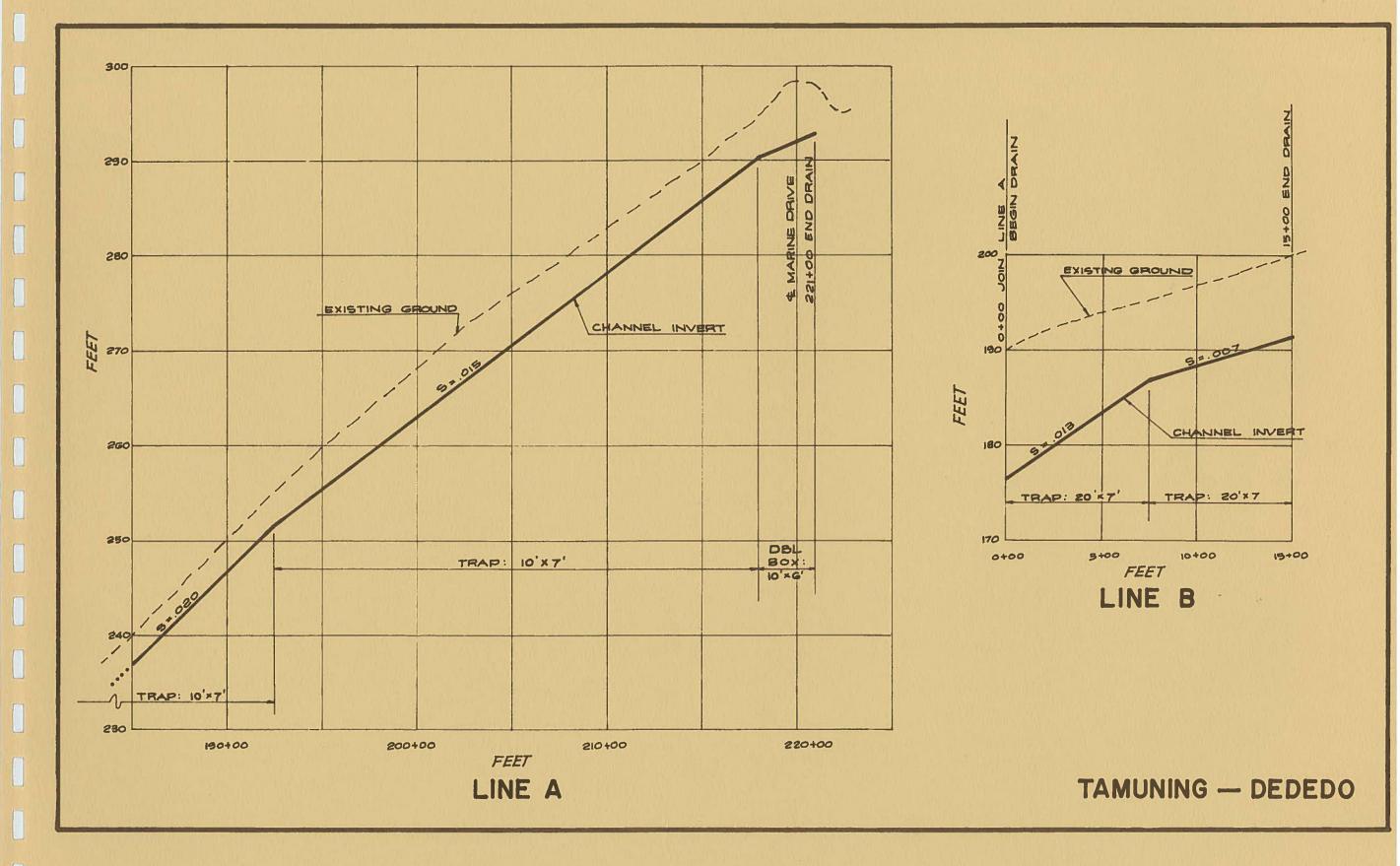


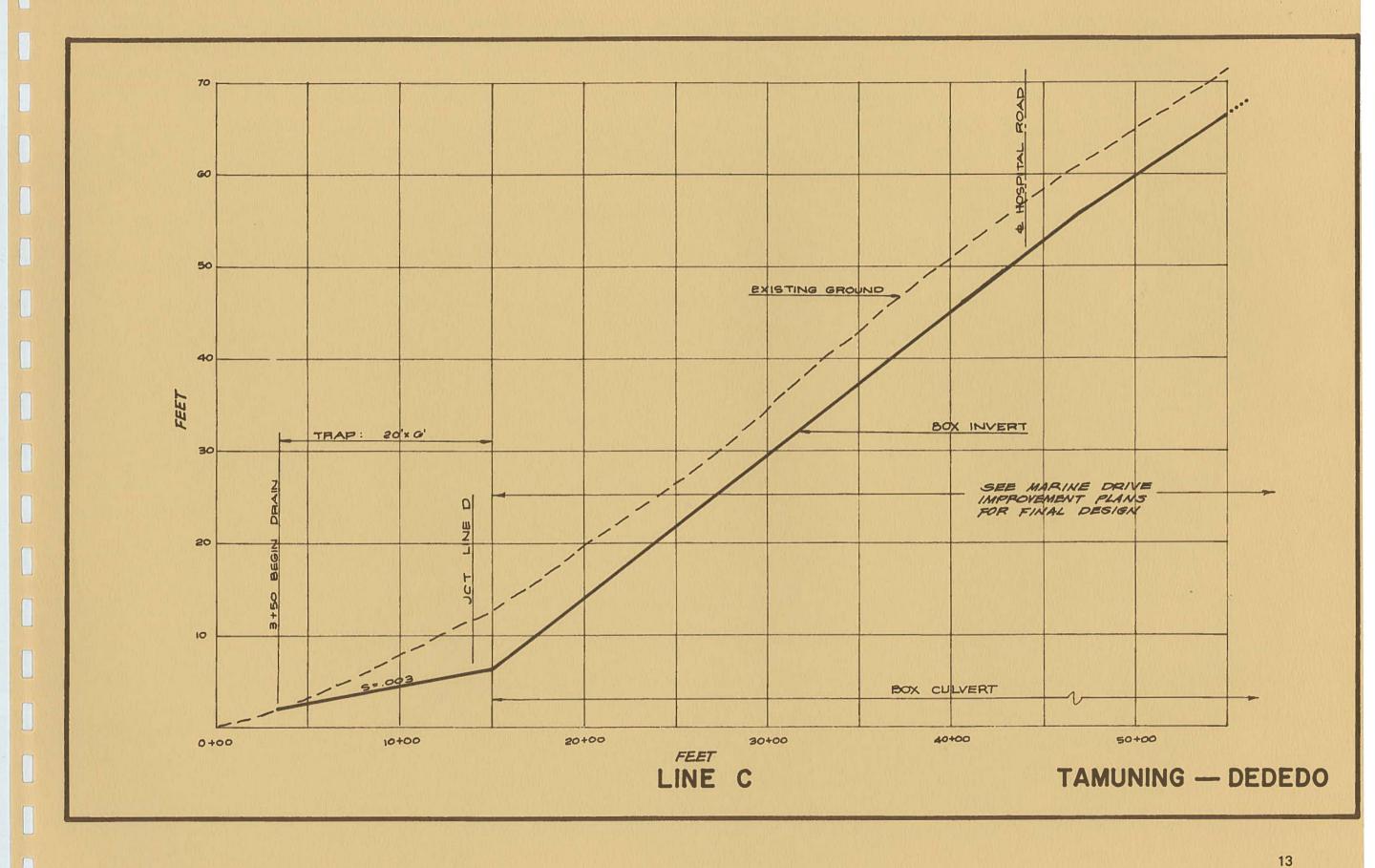


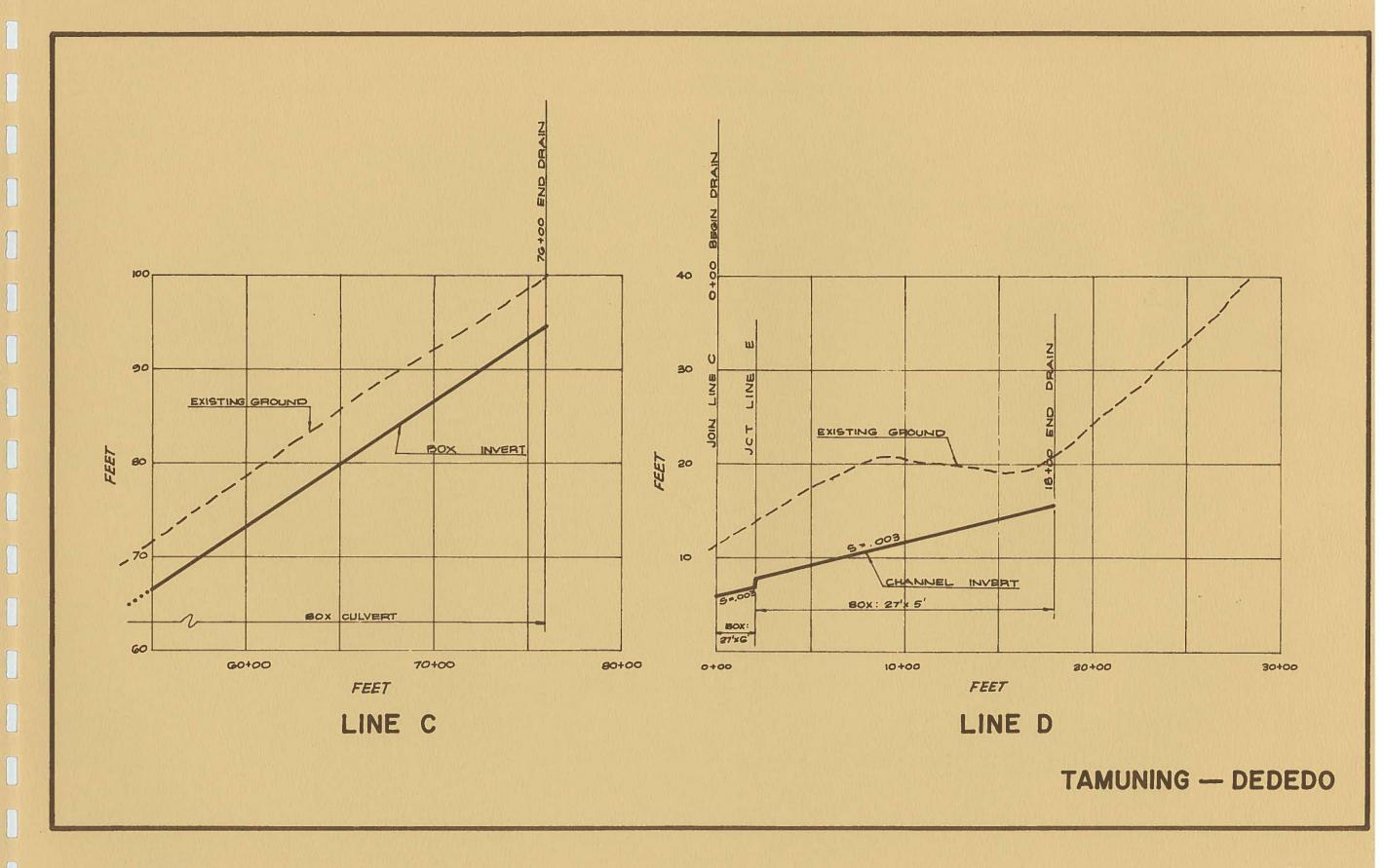


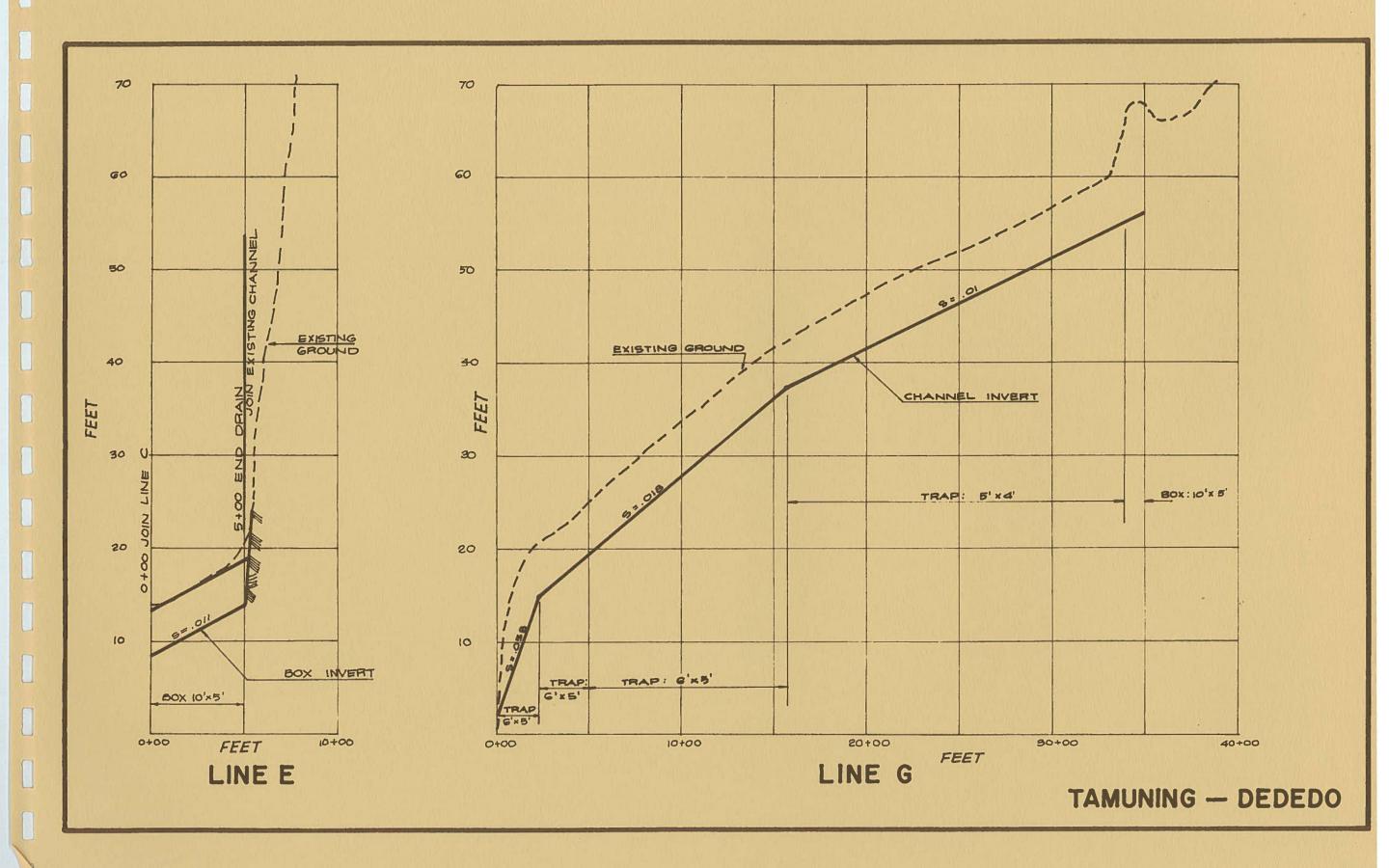


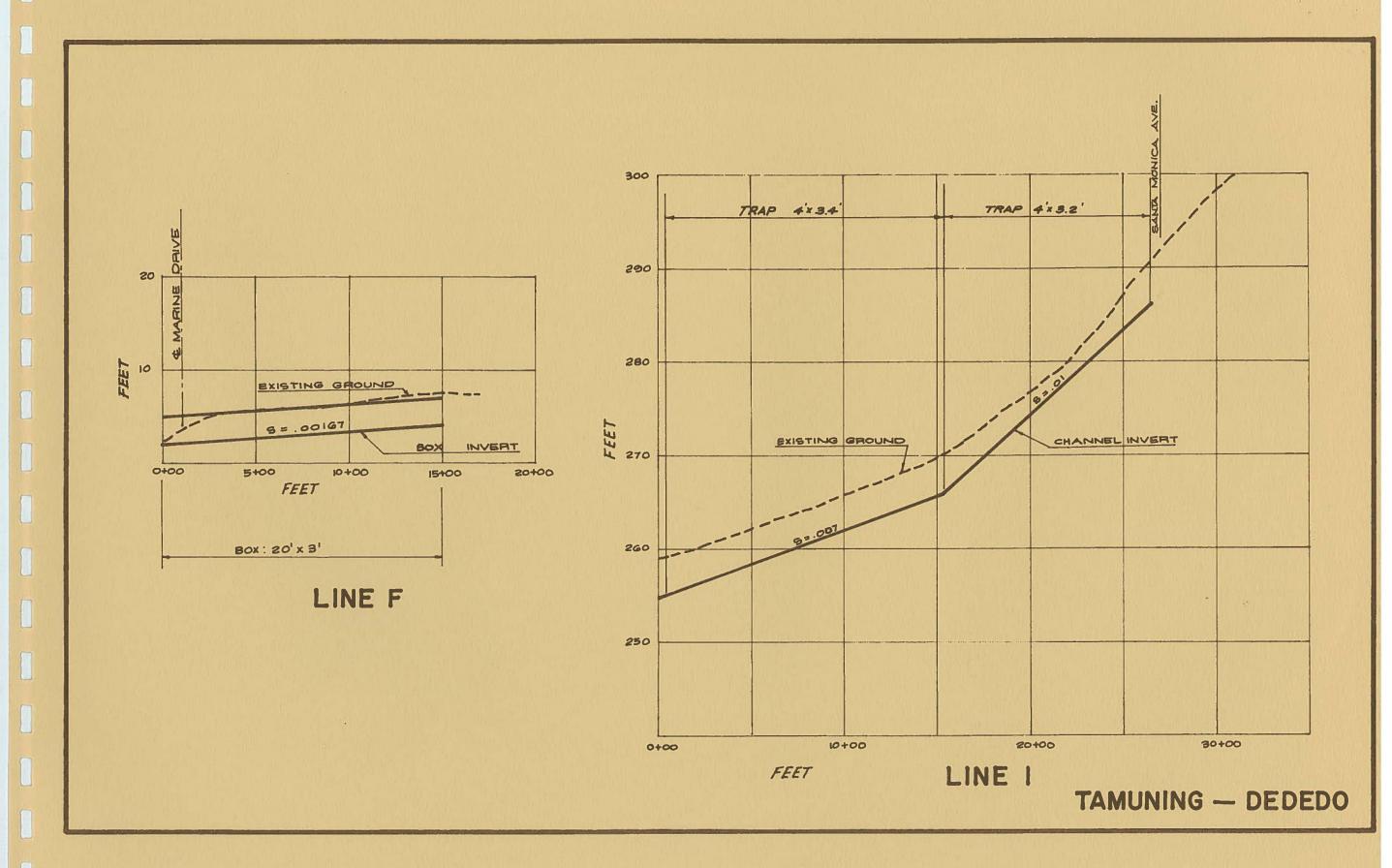


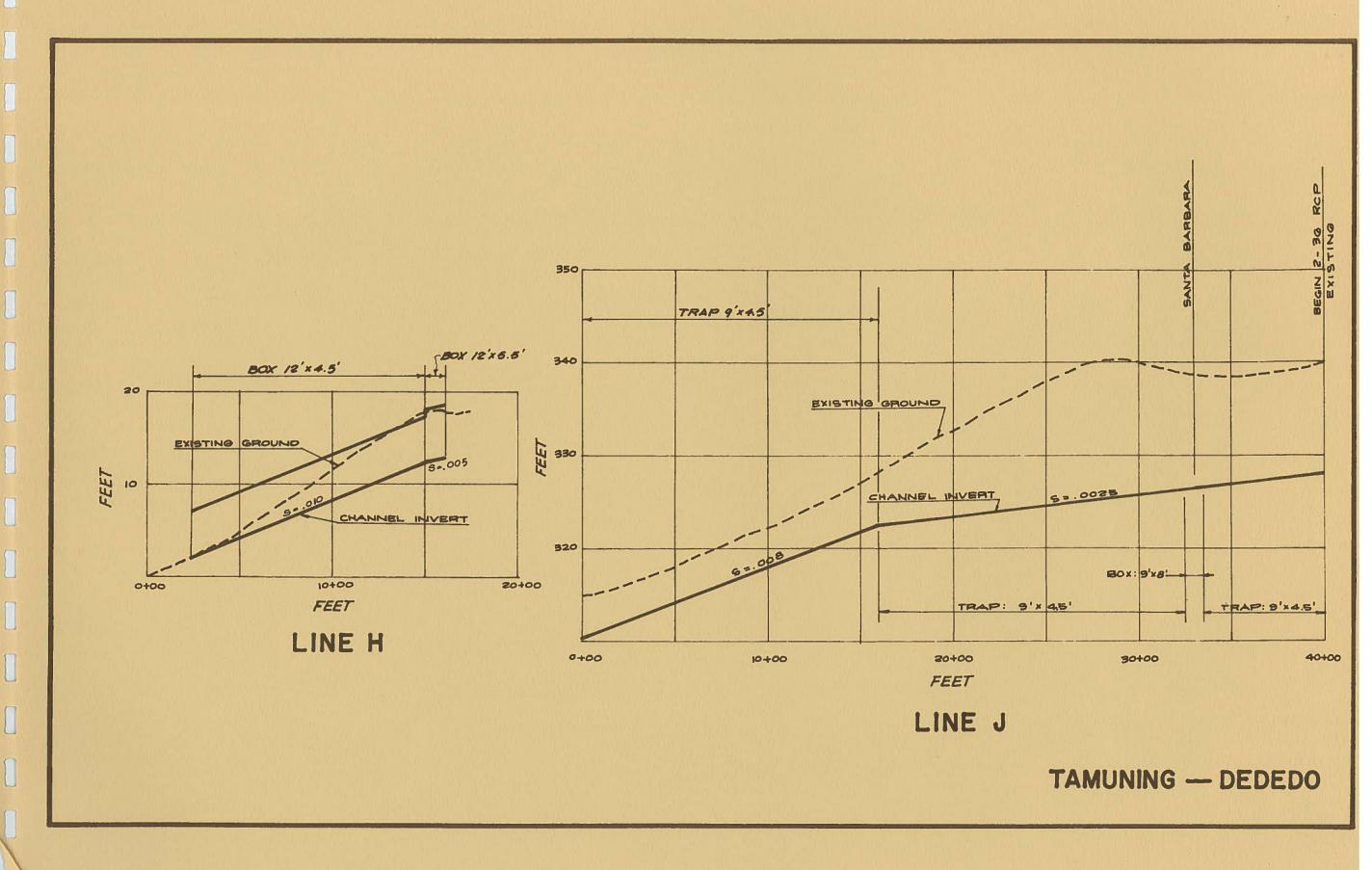












DRAINAGE TABULATION

TAMUNING - DEDEDO

LINE	DRA	NAGE A		С	CA	≥CA	1	Q I ≥ CA	SLOPE	n	Qn	TYPE	, L	V	tc	REMARKS
No.	NUMBER	ACRES	TOTAL ACRES	C	CA	20A			(FT/FT)		QII	SIZE	(FT)	(FT)	(MIN.)	REMARKS
A	A	202		.75	151	-	7.5	1130							23.0	Across Marine Dr
									.010	.013	15	DBL.Box 10x6	300	9	4.8	
									.015	.013	15	Trap 10 x 7	2600	9	4.6	
									.020	.013	15 15	Trap 10 x 7	2800	10	.8	
] .030	.013	13	Trap 10 x 7	550	12	33.5	
	A-1	460	662	.75	345	496	6.5	3220	.0025	.013	42	Trap 30 x 8	100	10	47.0	Vertical Walls
							ACME IN		.0025	.013	72	rrap so x o	100	10		
	A-2	550	1212	.70	385	881	6.4	5650								
	B, B-1	6730	7940	.50	1680	2561	2.9	7450 *	.0025	012	0.7		050		240.0	Across Route 16
									.0025	.013	97	TPL Box 14x10		15	1.9	
										2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2		Trap 35 x 9		16	2.4	
									.0030	013	97	Trap 35 x 9	2500	17	.6	
									.0200	.013	97	Trap 35 x 6		30	2.0	
									.012	.013	97	Trap 35 x 6		25	.2	
									.040	.013	97	Trap 35 x 5	500	40	247.4	
	A-3	960	9460	.75	720	3281	2.8	9200	.0025	012	120	F 54 0	000	2.4	1.1	
										.013	120	Trap 54 x 9		14	.5	Across Marine D
									.0025		120	DBL.Box 24x10	200	9	2.3	
	A-4	150	9610	.55	83	3364	2.8	9420	.0025		120			14	251.3	
									.0025	.013	122	DBL.Box 25x10				
									.055	.013	122	DBL.Box 23 x5		-		Discharge @ Tum
									.010	.013	122	DBL.Box 23 x8	700	-		Bay
В	В	6450		.50	1610		2.9	4660	007	07.0			1000000		240	
	B-1	280	6730	.55	70		2.9	4880 *	.007	.013	60	Trap 20 x 7 Trap 20 x 7		12		*Use R=.50 in v
							C. C. Carlotte		.013	.013	0.5	Tap ZU X /	730	14		of local depres-
APPLE		-														sions and storadareas available

DRAINAGE TABULATION TAMUNING - DEDEDO

LINE	DRA	INAGE A		С	CA	≥CA		Q I E CA	SLOPE		Qn	TYPE	L	V	(MIN.)	REMARKS
No.	NUMBER	ACRES	TOTAL ACRES	C	CA	ZUA			(FT/FT)	n	QII	SIZE	(FT)	(FT)	(MIN.)	ILMANNS
С	С	100	100	.60	60		6.2	372							15	Hosp.Rd.@MarineDr.
	C-1	100	200	.60	60	120	6.3	755							18,40	(See Marine Dr.
	D	286	486	.60	172	292	6.2	1810							37	Improvement Plans for Design)
	E	246	732	.55	135	427	6.2	2650	.003	.013	35	Trp. 20 x 6	1300	10	41	for besign,
-	C-2	10	742	.60	6	433	6.2	2680		.013	33	11p. 20 x 0	1300	10		
-										ALIEN IN				Charge.		
	D	206							Maria IIV						40	
D	D	286	-	.70	200		6.2	1240	.003	.013	16	Box 27 x 5	1600	9.8		
-	E	246	532	.70	172	372	6.0	2230	.003	.013	29	Box 27 x 6	200		43	
															THE WAY	
E	E	246		.65	160	-	6.5	1040					Child and		37	
		,				1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1			.011	.013	13	Box 10 x 5	500			
							Made									
F	F	57	-	.65	37		7.3	270	00167	012					15	
Marine Control									.00167	.013	4	Box 20 x 3	1500			
														Per management		
G	G	230	-	.70	161	-	7.7	1240	.01	.013	16	Box 10 x 5	100	17	20	
5									.01	.013	16	Trap 6 x 4	1800		2	
	G-1 G-2	95	326	.70	66	227	7.5	1700	.018	.013	22	Trap 6 x 5	1100	23	22	
	G-2	108	434	.70	76	303	7.4	2250	.018	.013	29	Trap 6 x 5	250		23	
									.058	.013	29	Trap 6 x 5	250	20		
Н	Н	115		.70	80		7.8	625							20	
	H-1	132	247	.70	92	172	7.8	1340	.005	.013	8	Box 12 x 4.5	100	12	20	
								to John	.010	.013	17	Box 12 x 5.5	1250	18		
								Trate India					AND THE PARTY			

DRAINAGE TABULATION

TAMUNING-DEDEDO

									10 DEDE							
LINE	DRA	INAGE A		С	CA	≥CA		Q I Z CA	SLOPE	n	Qn	TYPE	L (FT)	(ET)	†c	REMARKS
No.	NUMBER	ACRES	TOTAL		0/1				(FT/FT)			SIZE	(FI)	(FT)	(MIN.)	HEMANNO
I	I	67	-	.60	40	-	6.2	248	.01	.013	2 2	Mana A 2 2	1100	6	15	
									.007	.013	3.2	Trap 4 x 3.2 Trap 4 x 3.4	1100		5	
	<u>I-1</u>	127	194	.60	76	116	7.5	870			3.2	140 1 1 311	1300		23	
													Magazi			Outside project boundary
J	J	167	-	.60	100		7.9	790		010					20.0	Existing: 2-36"RCP
									.0025	.013	10	RCP: 2-36" Trap: 9x4.5'	750 2400		10	
	J-1	75	242	.60	45	145	6.0	870	.008	.013	11.0	Trap: 9x4.5	1600		30.3	
	J-2	275	517	.60	165	310	5.0	1550		.013	11.0	11ap. 7x4.5	1000	/2	34.0	
																Outside project boundary
PER	System (C)															
													in its same			
										Marine N						
											1					
Williams																
Maria in														red Mila	<u> Zanthu a</u>	
		المرازية والمرازية					2							THE PARTY	To the World	
														A TOTAL		

COST ESTIMATE

TAMUNING - DEDEDO

Line A		Line E	
Fence 198,500 Real Estate 160,820 Concrete 2,517,750 Excavation 544,700		Fence Real Estate (Easement) Concrete Excavation - 2,000 36,750 1,850	
Total for Line A	\$3,421,770	Total for Line E	\$ 40,600
Line B		Line F	
Fence 15,000 Real Estate 13,000 Concrete 75,960 Excavation 32,230		Fence Real Estate (Easement) 3,000 Concrete 112,500 Excavation 8,330	
Total for Line B	\$ 136,190	Total for Line F	\$ 123,830
Line C		Line G	
Fence 11,500 Real Estate 19,270 Concrete 328,310 Excavation 32,000		Fence 35,000 Real Estate 7,500 Concrete 136,800 Excavation 30,400	
Total for Line C	\$ 391,080	Total for Line G	\$ 209,700
Line D		Line H	
Fence Real Estate Concrete Excavation 4,860 198,600 18,660		Fence Real Estate 1,620 Concrete 130,650 Excavation 8,150	
Total for Line D	\$ 222,120	Total for Line H	\$ 140,420

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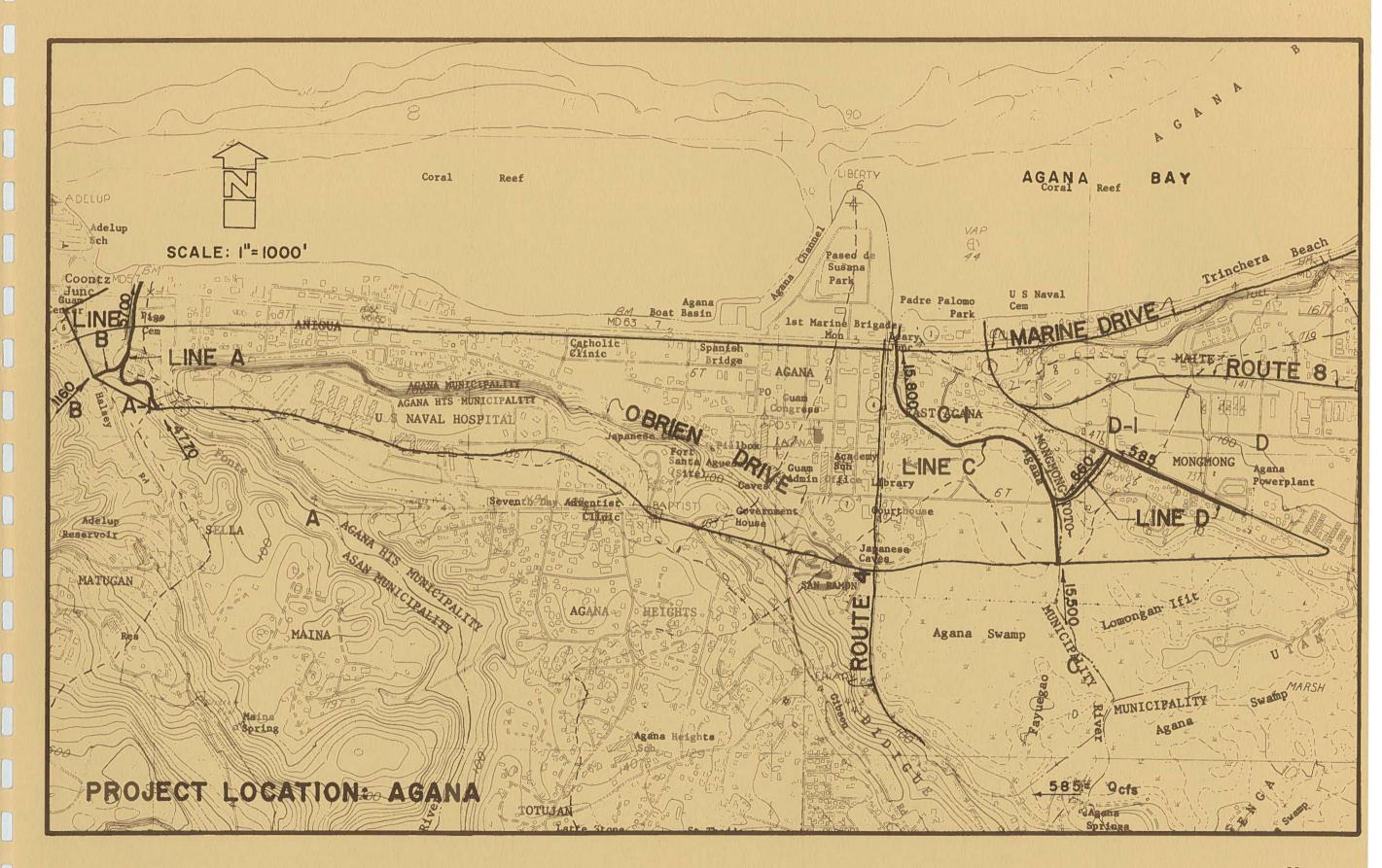
TAMUNING - DEDEDO

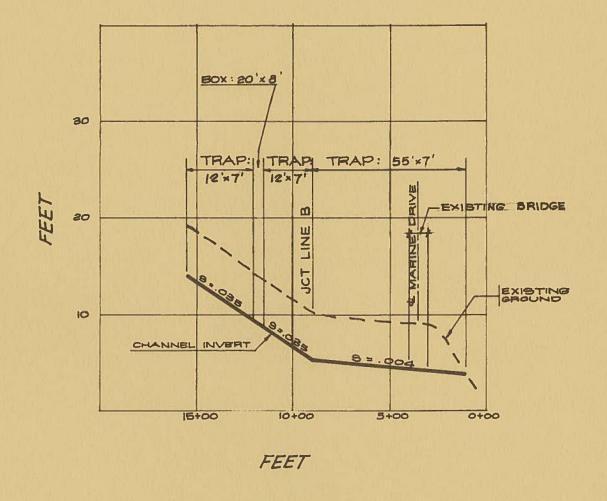
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Total

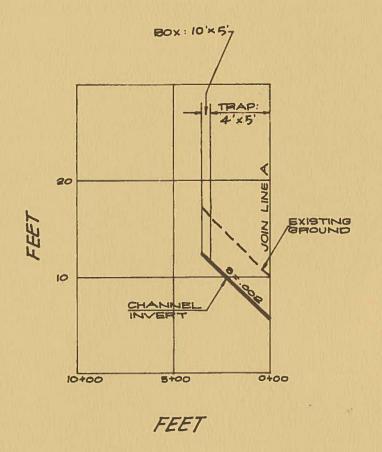
Lin	e 1			
	Fence	26,000		
	Real Estate	9,620		
	Concrete	72, 150		
	Excavation	8,420		
	Total for Line I		\$	116, 190
Lin	е Ј			
	Fence	40,000		
	Real Estate	18,800		
	Concrete	172,050		
	Excavation	30,000		
	Total for Line J		\$	260,850
	Sub-Total		5	,062,750
	10% Contingencies			506,270
	Construction Costs		5	, 569, 020
	10% Engineering & Inspectio	n	N. Illin	556,900

\$6,125,920



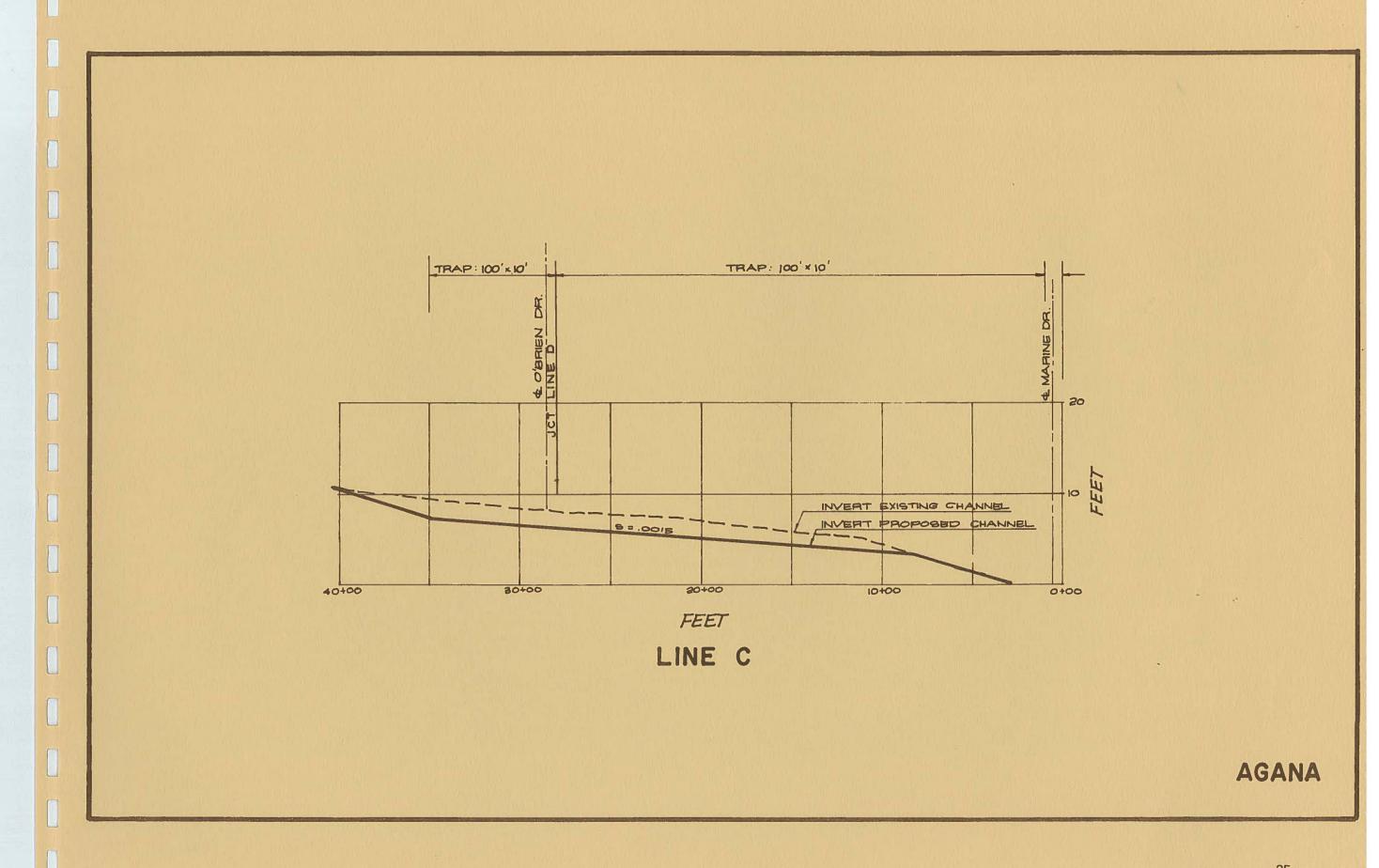


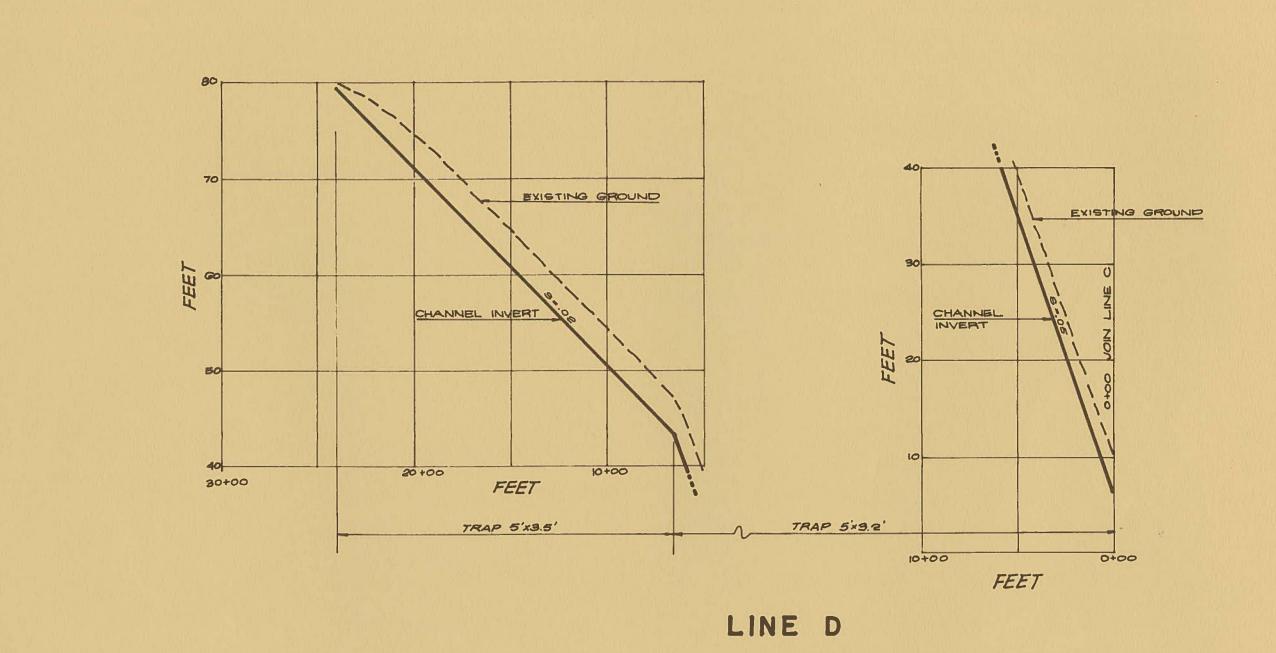
LINE A



LINE B

AGANA





AGANA

DRAINAGE TABULATION

LINE	DRA	INAGE A			C A	501		Q I≥CA	SLOPE		Qn	TYPE	L.	V	tc	REMARKS
No.	NUMBER	ACRES	TOTAL ACRES	С	CA	≥CA			(FT/FT)	n	ווע	SIZE	(FT)	(FT)	(MIN.)	REWARKS
A	A	1220	1220	.60	735	735	6.5	4770	.035	.013	62	Trap: 12' x6'	350	47	48	Fonte River
						y 115 W			.035	.013	62	Box: 20 x 8	50	 	48	
									.033	.013	02	BOX. 20 X 0	30	40		
	A-1, B	233	1453	.60	140	875	6.5	5700	.004	.013	74	Trap: 55 x 6	800	16	48	
															49	
В	В	226	226	.65	147	147	7.9	1160							20	
		MENT	220	. 05				1100	.02	.013	15	Box: 10' x 5'			20	
									.02	.013	15	Trap: 4 x 5°	300	23		
								L. J. Santier								
С	С	11900	11900	.50	5950	5950	2.6	15500	.0015	.013	202	Trap: 100 x10	700	10	300	Agana River
	D, D-1	150	12050	.50	75	6025	2.6	15700	.0015	.013		Trap: 100 x10		The second second second	301	
	C-1	75	12125	.65	49	6074	2.5	15800	.0015	.013	20 10000	Trap: 100 x10	The second secon		305	
D	D	124	124	.60	74	74	7.9	585							20	
	D-1	24	148	.60	14	88	7.5	660	.02	.013	8	Trap: 5 x 3.5	The state of the s		23	
									.05	.013	9	Trap: 5 x3.25	650	9		
																The second secon
TAN DE LA	rti						PENNINE.									
											Sala Bering					
					United by											